

Model:
TSSU-27-8

Food Prep Table: Solid Door Sandwich/Salad Unit



STANDARD FEATURES

DESIGN

- True's commitment to using the highest quality materials and oversized refrigeration systems provides the user with colder product temperatures, lower utility costs, exceptional food safety and the best value in today's food service marketplace.

REFRIGERATION SYSTEM

- Factory engineered, self-contained, capillary tube system using environmentally friendly (CFC free) 134A refrigerant.
- Oversized, factory balanced refrigeration system with guided airflow to provide uniform temperature in food pans and cabinet interior.
- Patented forced-air design holds 33°F to 41°F (.5°C to 5°C) product temperature in food pans and cabinet interior. Complies with and listed under ANSI/NSF-7-1997-6.3.
- Sealed, cast iron, self-lubricating evaporator fan motor and larger fan blades give True sandwich/salad units a more efficient, low velocity, high volume airflow design.
- Condensing unit access in back of cabinet, slides out for easy maintenance.

CABINET CONSTRUCTION

- Exterior - stainless steel front, top and ends. Matching aluminum finished back.
- Interior - attractive, NSF approved, white aluminum liner. 300 series stainless steel floor with coved corners.
- Insulation - entire cabinet structure and solid door are foamed-in-place using Ecomate. A high density, polyurethane insulation that has zero ozone depletion potential (ODP) and zero global warming potential (GWP).
- 5" (127 mm) diameter stem castors - locks provided on front set. 36" (915 mm) work surface height.

DOOR

- Stainless steel exterior with white aluminum liner to match cabinet interior.

- Door fitted with 12" (305 mm) long recessed handle that is foamed-in-place with a sheet metal interlock to ensure permanent attachment.
- Positive seal self-closing door with 90° stay open feature. Door swings within cabinet dimensions.
- Magnetic door gasket of one piece construction, removable without tools for ease of cleaning.

SHELVING

- Two (2) adjustable, heavy duty PVC coated wire shelves 23 1/4" L x 16" D (591 mm x 407 mm). Four (4) chrome plated shelf clips included per shelf.
- Shelf support pilasters made of same material as cabinet interior; shelves are adjustable on 1/2" (13 mm) increments.

MODEL FEATURES

- Evaporator is epoxy coated to eliminate the potential of corrosion.
- 11 3/4" (299 mm) deep, 1/2" (13 mm) thick, full length removable cutting board. Sanitary, high-density, NSF approved white polyethylene provides tough preparation surface.
- Stainless steel, patented, foam insulated lid and hood keep pan temperatures colder, lock in freshness and minimize condensation. Removable for easy cleaning.
- Comes standard with 8 (1/2 size) 6 7/8" L x 6 1/4" W x 4" D (175 mm x 159 mm x 102 mm) clear polycarbonate, NSF approved, food pans in countertop prep area. Also accommodates 6" (153 mm) and 8" (204 mm) deep food pans (supplied by others).
- Countertop pan opening designed to fit varying size pan configurations with available pan divider bars. Varying size pans supplied by others.
- NSF-7 compliant for open food product.

ELECTRICAL

- Unit completely pre-wired at factory and ready for final connection to a 115/60/1 phase, 15 amp dedicated outlet. Cord and plug set included.



115/60/1
NEMA-5-15R

OPTIONAL FEATURES/ACCESSORIES

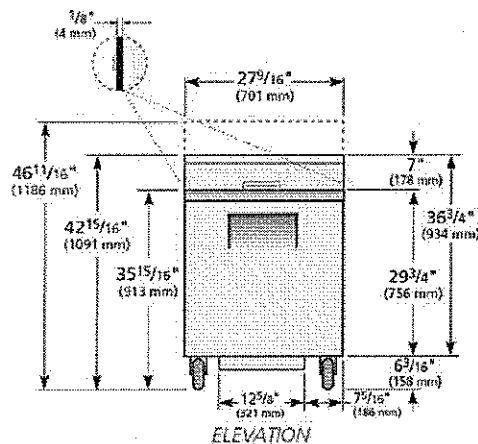
Upcharge and lead times may apply.

- ☐ 230 - 240V / 50 Hz.
- ☐ 6" (153 mm) standard legs.
- ☐ 6" (153 mm) seismic/flanged legs.
- ☐ 2 1/2" (64 mm) diameter castors.
- ☐ Barrel lock (factory installed).
- ☐ Additional shelves.
- ☐ Single overshef.
- ☐ Double overshef.
- ☐ Flat lid.
- ☐ Sneezeguard.
- ☐ 19" (483 mm) deep, 1/2" (13 mm) thick, white polyethylene cutting board. Requires "L" brackets.
- ☐ 19" (483 mm) deep, 3/4" (20 mm) thick, white polyethylene cutting board. Requires "L" brackets.
- ☐ 11 3/4" (299 mm) deep, 1/2" (13 mm) thick, composite cutting board. Requires "L" brackets.
- ☐ 19" (483 mm) deep, 1/2" (13 mm) thick, composite cutting board. Requires "L" brackets.
- ☐ Crumb catcher. Requires crumb catcher cutting board for proper installation.
- ☐ Pan dividers.
- ☐ Exterior rectangular digital temperature display (factory installed).
- ☐ Plexiglas lid cover.
- ☐ ADA compliant model with 34" (864 mm) work surface height.
- ☐ Remote cabinets (condensing unit supplied by others; system comes standard with 404A expansion valve and requires R404A refrigerant). Consult factory technical service department for BTU information. All remote units must be hard wired during installation.

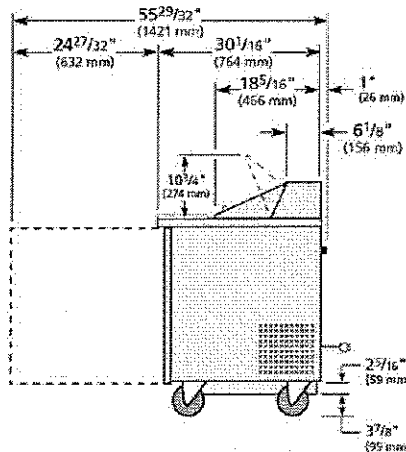
PLAN VIEW

Cubic Feet/Liters 6.5/184

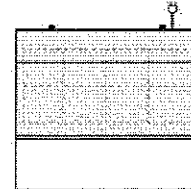
*Based on ANSI/AHAM HRF-1-2004



ELEVATION



RIGHT VIEW



PAN LAYOUT

WARRANTY

One year warranty on all parts and labor and an additional 4 year warranty on compressor. (U.S.A. only)

METRIC DIMENSIONS ROUNDED UP TO THE NEAREST WHOLE MILLIMETER

SPECIFICATIONS SUBJECT TO CHANGE WITHOUT NOTICE



Model	Elevation	Right	Plan	3D	Back
TSSU-27-8	TFNY01E	TFNY01S	TFNY01P	TFNY013	

TRUE FOOD SERVICE EQUIPMENT

2001 East Terra Lane • O'Fallon, Missouri 63366 • (636)240-2400 • Fax (636)272-2408 • (800)325-6152 • Intl. Fax# (001)636-272-7546 • www.truemfg.com

Commercial Building Permit Application



All Projects (please print or type)		DATE	PROJECT # (assigned by city)
OCCUPANT	NINFA'S	SUBMITTED BY	CONTACT PHONE 602.492.9494
PROJECT ADDRESS	2704 NAVIGATION BLVD	UNIT	CONTACT FAX 602.997.3229
CITY	HOUSTON	ZIP	77003
COUNTY	HARRIS	KEY MAP #	129-721
TYPE OF STRUCTURE ☐ RETAIL ☒ RESTAURANT ☐ OFFICE ☐ WAREHOUSE ☐ REPAIR ☐ GARAGE ☐ POOL ☐ FENCE ☐ OTHER			
SCOPE OF PROJECT (CHECK ONE) ☐ NEW CONSTRUCTION ☐ DEMOLITION ☐ ADDITION ☐ REPAIR ☐ REMOVAL ☐ FOUNDATION ☒ OTHER NEW BAR AT EXISTING PATIO			
PRESENT OCCUPANCY	RESTAURANT	# OF STORIES	1
PROPOSED OCCUPANCY	RESTAURANT		
TOTAL COST OF IMPROVEMENTS	\$ 20,000.00	APT. UNIT #S IN THIS BLDG. (EX. 101, 102, 103)	IDLR PROJECT # (CHECK ONE) ☐ PROJECT ☒ EXEMPT
GREEN BUILDING: TYPE (if applicable) ☐ LEED ☐ GBI			

New Construction and/or Addition Only (increased square footage)

SUBDIVISION	BLOCK/LOT	TRACT	SQ. FT. ADDED
SPRINKLERS: NO YES PERCENTAGE TYPE			

Responsible Parties (as applicable)

OWNER'S PROJECT MANAGER (required)	ADDRESS	CITY	ZIP	PHONE
BAR NAPKIN PRODUCTIONS, LLC. ON BEHALF OF LEGACY RESTAURANTS CONTACT: LAUREN GREEN	2944 N. 44TH ST. #144	PHOENIX, AZ	85018	602.492.9494
E-MAIL	LAUREN@BNP-LLC.COM; CHARTER@LEGACYRESTAURANTS.COM			
CONTRACTOR	ADDRESS	CITY	ZIP	PHONE
TBD				
AGENT	ADDRESS	CITY	ZIP	PHONE

Other Remarks

SCOPE OF WORK TO INCLUDE ONLY ADDING A BAR TO AN EXISTING PATIO OF AN EXISTING RESTAURANT. EXISTING RESTAURANT AND REMAINDER OF EXISTING PATIO NOT IN SCOPE AND TO REMAIN AS EXISTING.

Warning: The deed restrictions affidavit on the reverse side is a part of this application. Any false statement thereon may result in criminal prosecution. The City will rely upon the representations in the affidavit in issuing the permit. The issuance of a permit does not authorize construction on, or use of, any property in violation of deed restrictions. Any misrepresentations on the affidavit will render the permit void.

PRINT NAME OF APPLICANT

APPLICANT SIGNATURE

APPLICANT IS ☒ OWNER ☐ AGENT (DOCUMENT DESIGNATING AGENT'S AUTHORITY MUST BE ATTACHED)



Job No. 10-231

Sheet No. 1/11

By APB

Date 6-8-10

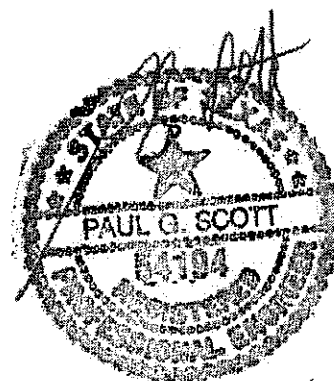
**CARUSO
TURLEY
SCOTT**
consulting
structural
engineers

CLIENT:

BAR NAPKIN PRODUCTIONS, LLC
2944 N. 44th Street, Suite #144
Phoenix, AZ 85018

PROJECT:

Ninfa's T.I.
2704 Navigation Blvd.
Houston, TX 77003



6/8/10

STRUCTURAL ENGINEERING
EXCELLENCE

PARTNERS

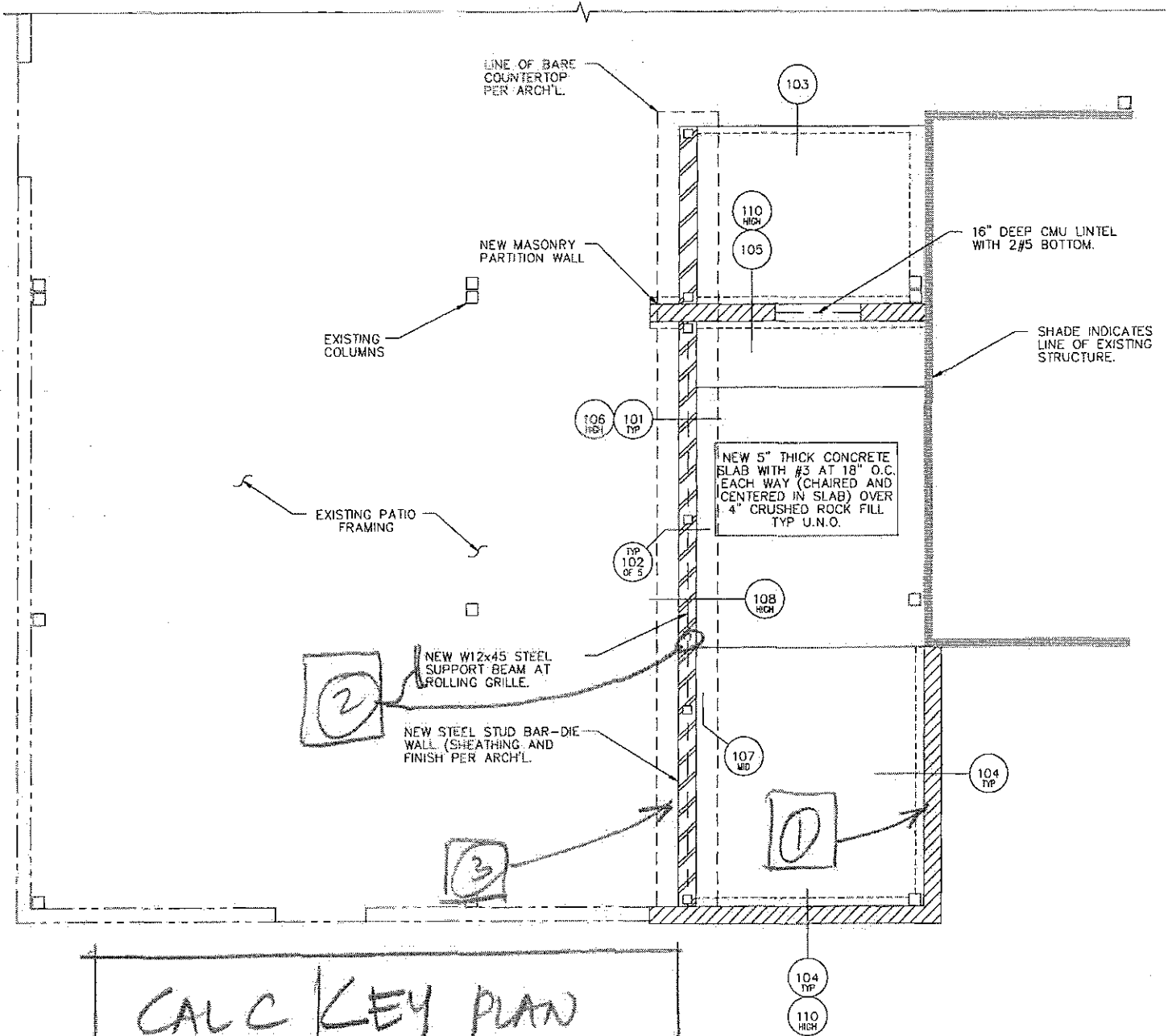
Richard D. Turley, PE
Paul G. Scott, PE, SE
Sandra J. Herd, PE, SE
Chris J. Atkinson, PE, SE
Thomas R. Morris, PE
Richard A. Dahlmann, PE

GENERAL INFORMATION:

BUILDING CODE:	2006 IBC/ ASCE 7-05
SOILS DATA:	Q = 1500 PSF min.
WIND:	90MPH Exp. C

1215 W. Rio Salado Pkwy.
Suite 200
Tempe, AZ 85281
T: (480) 774-1700
F: (480) 774-1701
www.ctsaz.com

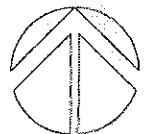
KEY PLAN

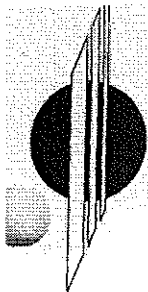


CALC KEY PLAN

ROLLING GRILLE SUPPORT &
FOUNDATION/PATIO FLOOR FRAMING PLAN

SCALE 1/4" = 1'-0"





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F: 480-774-1701

Job Name NINFA's T-1
Job No. 10-231 Sheet No. 2
By ATB Date 6/10

REQUIREMENTS:

① CMU ENCLOSURE WALL AT BAR AREA

WIND: 90 MPH EXP C

$F_w = 20.6 \text{ PSF}$
(see next page)

$F_w = 20.6 \text{ PSF}$

EXISTING WOOD ROOF
FRAMING AT PATIO BAR

ASSUMED
B'D MAX

* ENCLOSURE WALLS @ BAR

ARE NOT ATTACHED TO THE EXISTING
PATIO ROOF - ANALYZED AS CANTILEVERED
SITE WALL.

→ ENCLOSURE WALLS ARE INSIDE THE
PATIO AREA.

ok to me

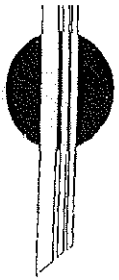
#5 @ 24" O.C. CENTERED IN WALL

CHECK FOOTING:

$$q = \frac{W_F}{15' \text{ wide}} = \frac{(70 \text{ PSF} \times 8') + 40 \text{ PSF}}{15'}$$

$$q = 493 \text{ PSF} < 1500 \text{ PSF (CODE minimum)}$$

ok! to me
16" WIDE CONC. FTG.



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130 South Priest Drive
Tempe, Arizona 85281
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F: 480-774-1701

Job Name _____
Job No. _____ Sheet No. 3
By APB Date 3/19/08

PER 2006 IBC / ASCE 7-05 (METHOD 2)

WIND: 90 MPH (3 SEC WIND GUST)
EXP. C

$$F = q_z \times G \times C_f \times A_p \quad (\text{EQUATION 6-27}) \\ \text{ASCE 7-05}$$
$$= 13.03 \times (0.85) (1.3 \times 11) (1 \text{ sq. ft}) \times 1.0$$

$$F = 20.60 \text{ kF}$$

↳ WIND PRESSURE

FOR EXP. C @ 90 MPH WIND.

PER
LOAD
COMBO
W=1.5

but

MINOR STRUCTURE
CATEGORY I (TABLE 6-1)

$$q_z = 0.00256 \times K_z \times K_{zt} \times K_d \times V^2 \times I$$
$$= 0.00256 \times (0.85) (1.0) (0.85) \times (90 \text{ MPH})^2 \times 0.87$$

EXP. C

$$q_z = 13.03$$

Caruso Turley Scott, Inc.
RIVERSIDE OFFICE PLAZA
1215 W. Rio Salado Pkwy, Ste200
Tempe, AZ 85281
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Title : Ninfa's Bar TI
Dsgnr: APB
Description :

Job # 10-231
Date: 10:14AM, 8 JUN 10

Scope :

Code Ref: ACI 318-02, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Rev: 580004
User: KW-0600624, Ver 5.8.0, 1-Dec-2003
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Cantilevered Retaining Wall Design

Page 1

10-231 - ninfa's ti - calculations.ecw.Calcul

Description 8'-0" Site Wall

Criteria

Retained Height = 0.17 ft
Wall height above soil = 8.00 ft
Slope Behind Wall = 0.00 : 1
Height of Soil over Toe = 2.00 in
Soil Density = 110.00 pcf
Wind on Stem = 20.6 psf

Soil Data

Allow Soil Bearing = 1,000.0 psf
Equivalent Fluid Pressure Method
Heel Active Pressure = 35.0
Toe Active Pressure = 0.0
Passive Pressure = 150.0
Water height over heel = 0.0 ft
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in

Footing Strengths & Dimensions

f_c = 3,000 psi F_y = 60,000 psi
Min. As % = 0.0014
Toe Width = 2.00 ft
Heel Width = 2.00
Total Footing Width = 4.00
Footing Thickness = 12.00 in
Key Width = 0.00 in
Key Depth = 0.00 in
Key Distance from Toe = 0.00 ft
Cover @ Top = 3.00 in @ Btm. = 3.00 in

Design Summary

Total Bearing Load = 1,160 lbs
...resultant ecc. = 7.23 in
Soil Pressure @ Toe = 552 psf OK
Soil Pressure @ Heel = 28 psf OK
Allowable = 1,000 psf
Soil Pressure Less Than Allowable
ACI Factored @ Toe = 773 psf
ACI Factored @ Heel = 39 psf
Footing Shear @ Toe = 6.9 psi OK
Footing Shear @ Heel = 1.0 psi OK
Allowable = 93.1 psi
Wall Stability Ratios
Overturning = 2.88 OK
Sliding = 2.38 OK
Sliding Calcs Slab Resists All Sliding !
Lateral Sliding Force = 188.8 lbs

Stem Construction

Design height ft = 0.00
Wall Material Above "Ht" = Masonry
Thickness = 8.00
Rebar Size = # 5
Rebar Spacing = 24.00
Rebar Placed at = Center

Top Stem

Design Data
 $f_b/FB + f_a/F_a$ = 0.509
Total Force @ Section lbs = 165.3
Moment....Actual ft-# = 687.2
Moment....Allowable = 1,349.8
Shear....Actual psi = 5.7
Shear....Allowable psi = 51.5
Bar Develop ABOVE Ht. in = 30.00
Bar Lap/Hook BELOW Ht. in = 6.23
Wall Weight = 61.0
Rebar Depth 'd' in = 3.81

Masonry Data

f_m psi = 1,500
 F_s psi = 24,000
Solid Grouting = No
Special Inspection = Yes
Modular Ratio 'n' = 21.48
Short Term Factor = 1.330
Equiv. Solid Thick. in = 5.20
Masonry Block Type = Normal Weight

Concrete Data

f_c psi =
 F_y psi =

Other Acceptable Sizes & Spacings

Toe: Not req'd, $M_u < S * F_r$
Heel: Not req'd, $M_u < S * F_r$
Key: No key defined

Footing Design Results

	Toe	Heel
Factored Pressure	773	39 psf
M_u : Upward	1,503	147 ft-#
M_u : Downward	553	266 ft-#
M_u : Design	950	118 ft-#
Actual 1-Way Shear	6.94	0.98 psi
Allow 1-Way Shear	93.11	93.11 psi
Toe Reinforcing	None Spec'd	
Heel Reinforcing	None Spec'd	
Key Reinforcing	None Spec'd	

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Title : Ninfa's Bar TI
Dsgnr: APB
Description :

Job # 10-231
Date: 10:14AM, 8 JUN 10

Scope :

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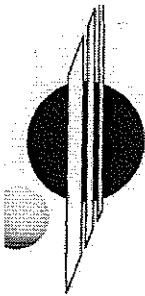
Cantilevered Retaining Wall Design

Page 2
10-231 - ninfa's ti - calculations.ecw;Calcul

Description 8'-0" Site Wall

Summary of Overturning & Resisting Forces & Moments

.....OVERTURNING.....				RESISTING.....					
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#		
Heel Active Pressure	=	24.0	0.39	9.3	Soil Over Heel	=	24.9	3.33	83.1
Toe Active Pressure	=				Sloped Soil Over Heel	=			
Surcharge Over Toe	=				Surcharge Over Heel	=			
Adjacent Footing Load	=				Adjacent Footing Load	=			
Added Lateral Load	=				Axial Dead Load on Stem	=		0.00	
Load @ Stem Above Soil	=	164.8	5.17	852.0	Soil Over Toe	=	36.7	1.00	36.7
SeismicLoad	=				Surcharge Over Toe	=			
					Stem Weight(s)	=	498.4	2.33	1,162.9
					Earth @ Stem Transitions	=			
					Footing Weight	=	600.0	2.00	1,200.0
					Key Weight	=			
					Vert. Component	=			
		</							

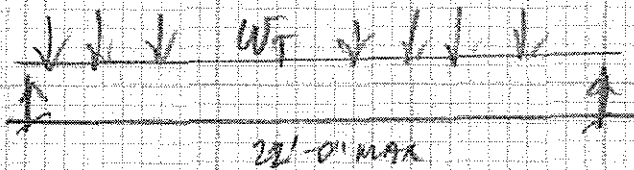


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Job Name NINFA T.I.
Job No. 10-231 Sheet No. 4
By AKB Date 4/10

② STEEL LINTEL @ BAR OPENING



$$W_T = \text{weight of open lintel above} + \text{wt. of rolling grille} \\ = (78 \text{ PSF} \times 5') + 350 \text{ PSF} \\ \text{Excessive solid gridded}$$

$$W_T = 400 \text{ PSF}_{\text{open}} + 350 \text{ PSF}_{\text{grille}}$$

$\therefore$ do use W12X45
STEEL LINTEL

Caruso Turley Scott, Inc.
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Title : Ninfa's Bar TI
Dsgnr: APB
Description :

Job # 10-231
Date: 10:14AM, 8 JUN 10

Scope :

Rev: 580002
User: KW-0600624, Ver 5.8.0, 1-Dec-2003
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Steel Beam Design

Page 1

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Description Steel Support Beam

General Information

Code Ref: AISC 9th ASD, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Steel Section : W12X45

Center Span 22.00 ft
Left Cant. 0.00 ft
Right Cant 0.00 ft
Lu : Unbraced Length 22.00 ft

Pinned-Pinned
Bm Wt. Added to Loads
LL & ST Act Together

Fy 50.00ksi
Load Duration Factor 1.00
Elastic Modulus 29,000.0 ksi

Distributed Loads

Note! Short Term Loads Are WIND Loads.

	# 1	# 2	# 3	# 4	# 5	# 6	# 7	
DL	0.400	0.350						k/ft
LL								k/ft
ST								k/ft
Start Location								ft
End Location								ft

Summary

Beam OK
Static Load Case Governs Stress

Using: W12X45 section, Span = 22.00ft, Fy = 50.0ksi
End Fixity = Pinned-Pinned, Lu = 22.00ft, LDF = 1.000

	Actual	Allowable		
Moment	48.088 k-ft	83.834 k-ft	Max. Deflection	-0.415 in
fb : Bending Stress	10.001 ksi	17.435 ksi	Length/DL Defl	636.0 : 1
fb / Fb	0.574 : 1		Length/(DL+LL Defl)	636.0 : 1
Shear	8.743 k	80.802 k		
fv : Shear Stress	2.164 ksi	20.000 ksi		
fv / Fv	0.108 : 1			

Force & Stress Summary

<<-- These columns are Dead + Live Load placed as noted -->>

	Maximum	DL Only	LL @ Center	LL+ST @ Center	LL @ Cants	LL+ST @ Cants	
Max. M +	48.09 k-ft	48.09					k-ft
Max. M -							k-ft
Max. M @ Left							k-ft
Max. M @ Right							k-ft
Shear @ Left	8.74 k	8.74					k
Shear @ Right	8.74 k	8.74					k
Center Defl.	-0.415 in	-0.415	0.000	-0.415	0.000	0.000 in	
Left Cant Defl	0.000 in	0.000	0.000	0.000	0.000	0.000 in	
Right Cant Defl	0.000 in	0.000	0.000	0.000	0.000	0.000 in	
...Query Defl @	0.000 ft	0.000	0.000	0.000	0.000	0.000 in	
Reaction @ Left	8.74	8.74		8.74			k
Reaction @ Rt	8.74	8.74		8.74			k

Fa calc'd per Eq. E2-2, $K \cdot L / r > C_c$

1 Beam, Major Axis, $L / r_T > (510,000 \cdot C_b / F_y)^{0.5}$, Fb per Eq. F1-7

1 Beam, Major Axis, Fb per Eq. F1-8, $F_b = 12,000 C_b A_f / (l \cdot d)$

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Title : Ninfa's Bar TI
Dsgnr: APB
Description :

Job # 10-231
Date: 10:14AM, 8 JUN 10

Scope :

Rev: 580002
User: KW-0600624, Ver 5.8.0, 1-Dec-2003
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Steel Beam Design

Page 2

10-231 - ninfa's ti - calculations.ecw:Calcul

Description Steel Support Beam

Section Properties W12X45

Depth	12.060 in	Weight	44.84 #/ft
Web Thick	0.335 in	Ixx	348.000 in4
Width	8.045 in	Iyy	50.000 in4
Flange Thick	0.575 in	Sxx	57.700 in3
Area	13.20 in2	Syy	12.400 in3
Rt	2.150 in	R-xx	5.150 in
Values for LRFD Design....		R-yy	1.950 in
J	1.310 in4	Zx	64.200 in3
Cw	1,650.00 in6	Zy	19.000 in3
		K	1.080 in

Part	Rev	Material	Quantity	Description	Check Steel Column
Part: 540003	Rev: 540003	Material: 540003	Quantity: 1.00	Description: Steel Column	Check Steel Column

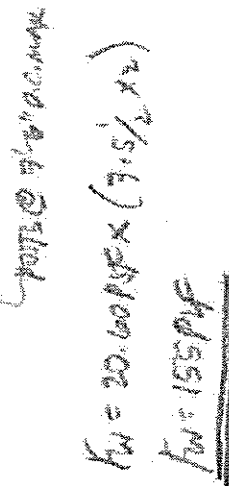
Section Properties		TS3.5X3.5X1/4		Values for LRFD Design...	
Depth	3.50 in	Weight	10.50 #/ft	J	8.820 in4
Thickness	0.250 in	Ixx	5.290 in4		0.00
Width	3.50 in	Iyy	5.290 in4	Zx	3.700 in3
		Sxx	3.020 in3	Zy	3.700 in3
Area	3.08 in2	Syy	3.020 in3		0.00
Perimeter	1.750 in	Box	1.310 in		0.00

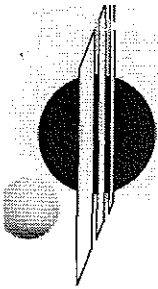
Section Type = TS-Square	1.316 in
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1114 CENTRE FOR THE STUDY OF THE HISTORY OF THE UNIVERSITY OF CAMBRIDGE

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CARUSO
TURLEY
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structural
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Job Name WIND: T1
Job No. 10-231 Sheet No. 10
By ME Date 6/10

③ CHECK WOOD LEDGER AT PATIO FLOORING @ NEW BATH ENCLOSURE

$$W = (C_{dw} + C_{wv}) \left(\frac{D}{L} \right) \times 2'-0" \text{ O.C.}$$

ANCHOR SPACING
@ LEDGER

$$R_n = 416 \# < 600 \# \text{ ALLOW} - 3/4" \phi \text{ ANCHOR @ } 24" \text{ O.C.} \leftarrow \text{GOVERNS!}$$

ON 3XB WOOD LEDGER

$$< 3250 \# - \text{CONCRETE ANCHORAGE}$$

ok to use

3XB WOOD LEDGER WITH 3/4" ϕ ANCHORS
SPACED AT 24" O.C.

BOLT DIA.	DOUG FIR-LARCH LEDGERS ₃			HEM FIR LEDGERS ₃			MASONRY LIMIT _{4,7}	CONCRETE LIMIT _{2,4,7}	EMBED. LENGTH
	2X	3X	4X	2X	3X	4X			
1/2"	370 610	---	---	320 550	---	---	850 6"	1250 3"	4"
5/8"	520 880	610 1000	700 1120	420 790	510 915	600 1040	1330 7.5"	2750 3.75"	4 1/2"
3/4"	590 1190	730 1405	870 1610	460 1020	600 1295	740 1490	1780 9"	3250 4.5"	5"
7/8"	630 1390	845 1890	1060 2190	500 1190	710 1705	920 1950	1920 10.5"	3700 5.25"	6"
1"	680 1590	985 2342	1290 2660	540 1360	840 2053	1140 2370	2050 12"	4125 6"	7"

1. VALUES ARE FROM 1991 NDS TABLE 8-2A.

2. $F'_c = 2,500$ PSI.

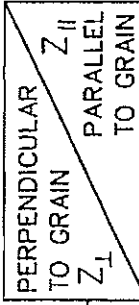
3. LOADS PER BOLT (LBS)

4. SECOND VALUE IS MINIMUM EDGE DISTANCE IN THE DIRECTION OF LOAD.

5. SEE APPLICABLE SECTIONS IN U.B.C. FOR BOLT SPACING IN WOOD, MASONRY AND CONCRETE.

6. VALUES MAY BE MULTIPLIED BY LOAD DURATION FACTORS WHERE APPLICABLE BUT SHOULD NOT EXCEED THE BOLT ALLOWABLE FOR THE APPROPRIATE MATERIAL (MASONRY OR CONCRETE).

7. EDGE DISTANCE PERPENDICULAR TO DIRECTION OF LOAD SHALL NOT BE LESS THAN 4".



EXAMPLE INTERPOLATION FOR 3/4" DIA. BOLT IN DOUG-FIR 3X MEMBER:

$$t_s = 2 \frac{1}{2}" \quad t_m = 2 \frac{1}{2}" \quad (2) = 5"$$

$$t_m = 4 \frac{1}{2}" \quad t_s = 1 \frac{1}{2}" \rightarrow Z_{\parallel} = 1200\# \quad t_s = 2 \frac{1}{2}" \rightarrow Z_{\parallel} = 1405\# \quad \text{FOR } t_m = 5"$$

$$t_s = 3 \frac{1}{2}" \rightarrow Z_{\parallel} = 1610\# \quad \text{AND } t_m = 2 \frac{1}{2}":$$

$$t_m = 5 \frac{1}{2}" \quad t_s = 1 \frac{1}{2}" \rightarrow Z_{\parallel} = 1200\# \quad t_s = 2 \frac{1}{2}" \rightarrow Z_{\parallel} = 1405\#$$

$$t_s = 3 \frac{1}{2}" \rightarrow Z_{\parallel} = 1610\# \quad \text{--- } Z_{\parallel} = 1405\#$$