EXISTING PATIO FLOOR LOADS: ASSUMED LIVE LOAD = 40 PSF (REDUCIBLE) ASSUMED DEAD LOAD = 12 PSF

GENERAL:

LOADS:

CONCRETE: SPECIFIED 28 DAY COMPRESSIVE STRENGTH F'C:

FOUNDATIONS AND GRADE BEAMS (DESIGN BASED ON 2,500 PSI)----- 3,000 PSI INTERIOR SLAB ON GRADE (NOT MOISTURE SENSITIVE) -- ---- 3,000 PSI

ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE ACI. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED UNLESS NOTED OTHERWISE. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. NO OTHER ADMIXTURES PERMITTED WITHOUT APPROVAL. FOR CONCRETE WITHOUT PLASTICIZER, MAXIMUM SLUMP 4 1/2" AT POINT OF PLACEMENT U.N.O. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL

FOR REINFORCING INFORMATION, SEE REINFORCING SECTION OF G.S.N., PLANS, SCHEDULES AND

UNLESS NOTED OTHERWISE ON THE DRAWINGS, THE EMBEDMENT OF CONDUITS, PIPES, SLEEVES, ETC. OF ANY MATERIAL SHALL NOT BE PERMITTED WITHIN ANY CONCRETE STRUCTURAL ELEMENT (IE: COLUMNS, BEAMS, ELEVATED SLABS, ETC.) OR STRUCTURAL CONCRETE TOPPINGS WITHOUT THE EXPRESSED APPROVAL OF THE STRUCTURAL ENGINEER.

TEST DATA FOR EACH CONCRETE MIX SHALL BE SUBMITTED FOR REVIEW PER CHAPTER 5 OF ACI 318. REFERENCE FIGURE R5.3 FOR SUBMITTAL REQUIREMENTS AND OPTIONS. CONCRETE MIX DESIGNS THAT ARE SUBMITTED WITHOUT THE APPROPRIATE TEST DATA CANNOT BE REVIEWED.

MAXIMUM SLUMP WITHOUT PLASTICIZER AT POINT OF PLACEMENT SHALL BE 5 INCHES. MIX DESIGNS SHALL TAKE CARE TO PROVIDE THE LARGEST POSSIBLE SIZE OF COURSE AGGREGATE WHILE MAINTAINING CONCRETE WORKABILITY. NOMINAL MAXIMUM AGGREGATE SIZE SHALL NOT BE LESS THAN 3/4 INCH NOR MORE THAN 1/3 THE DEPTH OF THE SLAB. MIX DESIGNERS SHALL SUBMIT SLAB ON GRADE DESIGNS WITH SHRINKAGE CHARACTERISTICS NOT EXCEEDING 0.00078 IN/IN TO MEET THE REQUIREMENTS OF ACI 360R-06, FIG5.6 FOR TYPICAL CONCRETE. SLABS SHALL BE PLACED ON A FLAT, SMOOTH, FIRM, COMPACTED SUBGRADE,

CONCRETE SHALL BE MIXED, PLACED, FINISHED AND CURED PER LATEST EDITION OF ACI 302.1 FOR THE APPROPRIATE FLOOR CLASS TYPE PER TABLE 1.1 AND SECTION 7. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR FINISH.

SLABS ON GRADE SHALL BE VIBRATED ONLY AT TRENCHES, FLOOR DUCTS, TURNDOWNS, ETC. CAST CLOSURE POUR AROUND COLUMNS AFTER COLUMN DEAD LOAD IS APPLIED.

### MASONRY:

GENERAL:

HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, MEDIUM WEIGHT, GRADE N, F'm = 1,500 PSI, RUNNING BOND, MORTAR TYPE S, 1,800 PSI. GROUT 2,000 PSI. MECHANICALLY VIBRATE GROUT IMMEDIATELY AFTER POURING AND AGAIN 5 TO 10 MINUTES LATER. PROVIDE CLEANOUTS IF GROUT LIFT EXCEEDS 5'-0" IN BLOCK WALLS. MAXIMUM GROUT LIFT SHALL BE 6'-0". WHEN APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING OFFICIAL, GROUT LIFTS MAY BE GREATER THAN 6'-O" IF IT CAN BE DEMONSTRATED BY CONTRACTOR THAT THE GROUT SPACES CAN BE PROPERLY FILLED. FILL CELLS SOLIDLY WITH GROUT IN LIFTS AND STOP POURS 1 1/2" BELOW THE TOP OF A COURSE TO FORM A KEY AT POUR POINTS. UNLESS NOTED OTHERWISE ON HE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUNS OF WALL EXCEEDS 24'-0". CONTROL JOINTS SHALL NOT OCCUR AT WALL CORNERS, INTERSECTIONS, ENDS, WITHIN 24" OF CONCENTRATED POINTS OF BEARING OR JAMBS, OR OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID.

SOLID GROUT SHALL BE PROVIDED BETWEEN WEBS AND MASONRY FACE SHELLS FOR FULL LENGTH OF ALL STEEL LINTELS. MORTAR MAY BE USED FOR GROUT FOR THIS PURPOSE ONLY. FACE UNITS, SOAPS, ROMANS, ETC., SHALL BE LAID WITH FULL HEAD AND BED JOINTS.

ALL REINFORCING PER CRSI SPECIFICATIONS AND HANDBOOK. ASTM A615 (Fy = 60 KSI / GRADE 60) DEFORMED BARS FOR ALL BARS #5 AND LARGER. ASTM A615 (Fy = 40 KSI / GRADE 40) DEFORMED BARS FOR ALL BARS #4 AND SMALLER. WHERE SHOWN ON DRAWINGS ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706. WELDED WIRE FABRIC PER ASTM A185, WIRE PER ASTM A82. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ----- 3" ALL OTHER PER LATEST EDITION OF ACI 318

ALL REINFORCING SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES. SUPPORT OF FOUNDATION REINFORCING MUST PROVIDE ISOLATION FROM MOISTURE/CORROSION BY USE OF A PLASTIC OR CONCRETE CHAIR. DUCT-TAPE COVERED REINFORCING IS NOT AN ACCEPTABLE CHAIR.

ALL DIMENSIONS REFERENCED IN DRAWINGS AS "CLEAR" SHALL BE FROM FACE OF STRUCTURE TO EDGE OF REINFORCING, AND SHALL NOT BE LESS THAN STATED, NOR GREATER THAN "CLEAR" DIMENSION PLUS 3/8". ALL OTHERS SHALL BE PLUS OR MINUS 1/4" TYPICAL UNLESS NOTED

FIELD BENDING OR STRAIGHTENING OF DEFORMED BARS SHALL BE LIMITED TO #5 BARS AND SMALLER AND SHALL BE FIELD BENT OR STRAIGHTENED ONLY ONCE. ANY BEND SHALL BE LIMITED TO 90 DEGREES. IF FIELD BENDING OR STRAIGHTENING OF #6 BARS OR LARGER IS REQUIRED, OR IF A SECOND BEND IS REQUIRED FOR #5 BARS AND SMALLER, "HEAT SHALL BE APPLIED FOR BENDING OR STRAIGHTENING. CONTRACTOR SHALL SUBMIT PROCEDURE FOR APPLYING HEAT TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO BENDING OR STRAIGHTENING BARS. LAP SPLICES IN CONCRETE:

REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. DOWEL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90-DEGREE HOOKS UNLESS NOTED OTHERWISE. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. ONLY WHEN SPECIFICALLY NOTED ON DRAWINGS MAY CONCRETE COLUMN DOWEL EMBEDMENT BE A STANDARD COMPRESSION DOWEL WITH EMBEDMENT LENGTH ACCORDING TO THE LATEST EDITION OF THE ACI 318

LAP SPLICES, UNLESS NOTED OTHERWISE, SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318. STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH. ONLY WHEN SPECIFICALLY NOTED ON DRAWINGS MAY LAP SPLICES IN CONCRETE COLUMNS BE STANDARD COMPRESSION LAP SPLICES. DRYPACK:

DRYPACK SHALL BE 5,000 PSI NON-SHRINK GROUT, FIVE STAR OR EQUIVALENTS. INSTALL DRYPACK UNDER BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS, INSTALL DRYPACK UNDER BASE PLATES AFTER COLUMN HAS BEEN PLUMBED BUT PRIOR TO SUPPORTED RAMING BEING INSTALLED

# STRUCTURAL STEEL:

ALL CONSTRUCTION PER LATEST AISC HANDBOOK. ALL TUBE STEEL SHALL BE ASTM A500 (Fy = 46 KSI). ALL MISCELLANEOUS STEEL UNLESS NOTED OTHERWISE SHALL BE ASTM A36 (Fy = 36 THE TERMS PIPE AND ROUND HOLLOW STRUCTURAL SHAPE (HSS) ARE USED SYNONYMOUSLY THROUGHOUT THESE DOCUMENTS ALONG WITH THE TERMS TUBE STEEL AND RECTANGULAR OR

ALL STRUCTURAL ROLLED STEEL MEMBERS WITH FY GREATER THAN 36 KSI ARE TO BE IDENTIFIED

UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE ASTM A307. ALL EXPANSIVE ANCHORAGE FOR CONCRETE INSTALLATION ONLY SHALL BE PER ITW/REDHEAD "TRUBOLT" WEDGE ANCHOR (ICC ESR-2251) OR APPROVED EQUIVALENT. ALL EXPANSIVE ANCHORAGE FOR MASONRY INSTALLATION ONLY SHALL BE PER SIMPSON "WEDGE-ALL" ANCHOR (ICC ESR-1396) OR APPROVED EQUIVALENT. ALL ADHESIVE (EPOXY) ANCHORAGE FOR CONCRETE OR MASONRY SHALL BE PER SIMPSON 'SET' SYSTEM WITH DUAL SIDE BY SIDE CARTRIDGES (ICC ESR-1772) OR APPROVED EQUIVALENT. ALL REFERENCE TO HEADED STUDS SHALL BE "TRW/NELSON" HIGH STRENGTH HEADED STUDS OR APPROVED EQUIVALENT. AT CONTRACTOR'S OPTION HEADED STUDS PER ABOVE MAY BE SUBSTITUTED FOR CONVENTIONAL ANCHORS AND MACHINE BOLTS (REVERSE SUBSTITUTION NOT ALLOWABLE). ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS AT SHORT SLOTTED HOLES USING SNUG TIGHT INSTALLATION UNLESS NOTED OTHERWISE.

STEEL ERECTION NOTE: PER OSHA, STEEL MEMBERS AND DIAGONAL BRACING CANNOT BE RELEASED FROM HOISTING CABLES UNTIL ALL BOLTS OR WELDS AT MEMBER ENDS ARE COMPLETE.

WITH AN ASTM SPECIFICATION MARK OR TAG PER IBC SEC. 2203.1

WELDING: UNLESS NOTED OTHERWISE, ALL WELDS PER LATEST EDITION OF THE AWS STANDARDS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. HESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS: THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

# COLD FORMED STRUCTURAL STEEL FRAMING:

ALL COLD FORMED STEEL COMPONENTS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE PER SSMA. COMPONENTS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE WITH THE LATEST EDITION OF 'SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" BY THE AISI AND ICC ER-4943P OR APPROVED EQUIVALENT. STEEL FOR COLD FORMED MEMBERS SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI FOR 43 MIL THICKNESS AND LESS, AND MINIMUM YIELD STRENGTH OF 50 KSI FOR 54 MIL THICKNESS AND GREATER. STEEL FOR BRIDGING SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI. STEEL SHALL BE GALVANIZED AT LOCATIONS EXPOSED TO WEATHER AND WHEREVER NOTED.

WHERE WELDING IS SHOWN ON DRAWINGS IT SHALL BE PERFORMED BY WELDERS EXPERIENCED IN LIGHT GAGE STRUCTURAL STEEL FRAMING WORK. DO NOT NOTCH FLANGES OF MEMBERS WITHOUT EXPRESSED APPROVAL OF THE ENGINEER OF RECORD.

COLD FORMED TRACKS AND LEDGERS BOLTED TO OTHER MATERIALS SHALL BE INSTALLED WITH MINIMUM 1/4" X 3" X 3" PLATE WASHERS.

SCREWED COLD FORMED TO COLD FORMED CONNECTIONS (I.E.: 97 MIL TO 97 MIL MAX) SHALL BE HEX HEAD HILT! "KWIK-PRO" SELF-DRILLING TAPPING SCREWS PER ICC ESR-2196, OR ICC APPROVED EQUIVALENT. SCREW SIZE SHALL BE #10 X 3/4" MINIMUM AND MUST PROTRUDE THROUGH AND BEYOND THE CONNECTED STEEL MEMBERS A MINIMUM OF THREE FULL THREADS SCREWS SHALL BE INSTALLED WITH A MINIMUM EDGE AND END DISTANCE OF THREE SCREW

### MIL THICKNESS / GAGE NUMBER REFERENCE:

27 MIL ---- 22 GA 54 MIL ---- 16 GA 33 MIL ---- 20 GA 68 MIL ----- 14 GA 43 MIL ---- 18 GA 97 MIL ----- 12 GA

STRUCTURAL STEEL STUD WALLS SHOWN ON PLAN SHALL BE INSTALLED PER STUD/SPACING AS NOTED BELOW, AND BOTTOM TRACKS SHALL BE INSTALLED WITH 1/2" DIA ANCHOR BOLTS AT 2'-8" O.C. TYPICAL U.N.O. ATTACHMENT OF STUDS TO TRACKS PER TYPICAL DETAILS U.N.O. ALL STUDS SHALL BE SECURELY SEATED FOR FULL END BEARING ON TOP AND BOTTOM TRACK. UNLESS NOTED OTHERWISE. INSTALL DOUBLE STUDS AT ALL JAMBS, CORNERS, INTERSECTIONS AND ISOLATED BEARING POINTS OF FRAMING MEMBERS ABOVE. BRIDGING SHALL BE INSTALLED PER

FOR AXIAL LOAD BEARING WALLS, INSTALL BRIDGING PER TYPICAL DETAILS. FOR WALLS WITH NO AXIAL LOAD, INSTALL BRIDGING AT MID-HEIGHT FOR WALLS LESS THAN OR EQUAL TO 10'-0" HIGH, AND 5'-0" O.C. MAXIMUM FOR WALLS GREATER THAN 10'-0" HIGH. IN ADDITION. BRIDGING SHALL BE INSTALLED AT ROOF LINES AND ELSEWHERE AS NOTED ON THE DRAWINGS. SOLID BLOCKING SHALL BE INSTALLED IN LIEU OF BRIDGING WHERE NOTED ON THE DRAWINGS. FOR NON-BEARING PARTITIONS AND SOFFITS, SEE ARCHITECTURAL DRAWINGS.

SSMA STRUCTURAL STEEL STUDS AND TRACKS SHALL BE AS FOLLOWS - TYPICAL U.N.O.:

MANUFACTURER'S RECOMMENDATION WITH THE FOLLOWING MINIMUM REQUIREMENTS:

STUD SIZE-MILS SPACING TRACK SIZE-MILS 3 5/8" - 362S162-43 362T125-54 -400S162-43400T125-54 - 600S162-43

FOR ADDITIONAL INFORMATION, SEE ARCHITECTURAL DRAWINGS.

SSMA NON-STRUCTURAL/NON-BEARING INTERIOR STEEL STUDS AND TRACKS SHALL BE AS FOLLOWS - TYPICAL U.N.O.: (DEŚIGN BASIS = 5 PSF INTERIOR LOAD, DEFLECTION LIMIT = L/240).

MAX WALL HEIGHT STUD SIZE-MILS STUD SPACING TRACK SIZE-MILS 362T125-27 3 5/8" 362S125-27 3 5/8" 362S125-27 12'-9" TO 14'-6" 362T125-27 14'-7" TO 19'-6" 6" 600S125-27 600T125-27 19'-7" TO 22'-4" 6" 600S125-27 22'-5" AND GREATER - NOTIFY STRUCTURAL ENGINEER

#### SHOP DRAWINGS:

REVIEW. UNLESS NOTED ACCORDINGLY.

SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS.

THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON CONTRACTOR'S REVIEW. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND FIELD CONDITIONS. MANUFACTURER OR FABRICATOR SHALL CLOUD ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS. ANY OF THE AFOREMENTIONED WHICH ARE NOT CLOUDED OR

FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED AFTER ENGINEER'S

THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.

THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT SHALL NOT BE CONSIDERED CHANGES TO CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE ITEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS

THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, CONTRACTOR SHALL

BE RESPONSIBLE FOR ALL NECESSARY CHANGES, APPROVALS AND THE COORDINATION OF THE

# SPECIAL INSPECTION - STRUCTURAL ONLY:

WORK WITH ALL RELATED TRADES AND SUPPLIERS.

FOR SPECIAL STRUCTURAL INSPECTIONS, CONTACT CARUSO TURLEY SCOTT, INC. AT 480-774-1700

SPECIAL INSPECTIONS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A STATE REGISTERED STRUCTURAL ENGINEER WHO IS FAMILIAR WITH THE STRUCTURAL DESIGN OF THIS PROJECT. THE SUPERVISING STRUCTURAL ENGINEER SHALL SEAL THE SPECIAL INSPECTION

SPECIAL INSPECTION IS TO BE PROVIDED FOR THE ITEMS LISTED BELOW IN ADDITION TO THE INSPECTIONS CONDUCTED BY THE BUILDING JURISDICTION. "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM REQUESTING THE BUILDING JURISDICTION INSPECTIONS REQUIRED BY SECTION 109 OF THE INTERNATIONAL BUILDING CODE. SPECIAL INSPECTION IS REQUIRED PER CHAPTER 17 FOR THE FOLLOWING:

# CONCRETE CONSTRUCTION:

DURING THE TAKING OF TEST SPECIMENS. CONTINUOUS INSPECTION DURING THE PLACEMENT OF ALL REINFORCED CONCRETE, UNLESS NOTED OTHERWISE. C. CONTINUOUS INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING THE PLACEMENT OF CONCRETE AROUND BOLTS. (EXCEPTION: NO INSPECTION IS REQUIRED FOR PLACEMENT OF CONCRETE AROUND FOUNDATION ÀNCHOR BOLTS.) D. NO INSPECTION IS REQUIRED FOR PLACEMENT OF SLAB ON GRADE CONCRETE. INSPECTION OF SLAB ON GRADE REINFORCING IS REQUIRED PER "REINFORCING STEEL" SECTION BELOW. NO INSPECTION IS REQUIRED FOR THE PLACEMENT OF FOUNDATION CONCRETE. INSPECTION OF FOUNDATION REINFORCING IS REQUIRED PER "REINFORCING STEEL" SECTION BELOW.

REINFORCING STEEL: INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE FOR THE FOLLOWING: A. REINFORCING FOR ALL CONCRETE REQUIRED TO HAVE INSPECTION NOTED ABOVE. REINFORCING FOR CONCRETE FOUNDATIONS. REINFORCING FOR SLABS ON GRADE.

REINFORCING FOR ALL MASONRY REQUIRED TO HAVE INSPECTION NOTED BELOW. 3. EXPANSION, EPOXY, AND ADHESIVE ANCHORS: DURING THE PLACEMENT OF ALL ANCHORS SHOWN ON STRUCTURAL DRAWINGS. ADDITIONAL INSPECTIONS REQUIRED FOR REPAIR DETAILS SHALL BE PERFORMED AT THE CONTRACTOR'S EXPENSE.

INSPECTION OF HOLE DIAMETER AND DEPTH INSPECTION OF HOLE CLEANING WITH WIRE BRUSH AND COMPRESSED AIR. INSPECTION OF ANCHOR INSTALLATION USING SPECIFIED PRODUCT AND MANUFACTURER'S RECOMMENDED INSTALLATION PROCEDURES.

## STEEL CONSTRUCTION:

WELDING: . PERIODIC VISUAL INSPECTION OF ALL FIELD WELDS. . CONTINUOUS INSPECTION OF ALL MULTIPASS FILLET WELDS OR SINGLE PASS FILLET WELDS . NON-DESTRUCTIVE TESTING OF ALL COMPLETE PENETRATION WELDS BY AN AWS CERTIFIED NDEPENDENT TESTING LABORATORY AT THE CONTRACTORS EXPENSE.

VERIFICATION OF VALID WELDER'S CERTIFICATES. ALL STRUCTURAL STEEL FABRICATORS SHALL EMPLOY AN AWS CERTIFIED INDEPENDENT TESTING LAB TO PROVIDE SHOP WELD INSPECTIONS PER CODE. INSPECTION REPORTS SHALL BE SUBMITTED TO ENGINEER OF RECORD PRIOR TO STEEL INSTALLATION. EXCEPTION: NO SHOP INSPECTION IS REQUIRED IF THE FABRICATOR IS ON THE CITY OF PHOENIX APPROVED STEEL FABRICATOR LIST. MASONRY CONSTRUCTION:

STRUCTURAL MASONRY:

WORKMANSHIP PROVISIONS OF THE CODE.

DURING PREPARATION OF PRISMS. INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE DELIVERY OF GROUT CONTINUOUS INSPECTION DURING PLACEMENT OF GROUT. . CLEANOUTS PRIOR TO CLOSING.

# DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATION. B. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE DESIGN DRAWINGS OR SPECIFICATIONS, AND ALL DEVIATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO PROCEEDING WITH THE WORK. ALL REQUESTS FOR DEVIATIONS SHALL BE INITIATED BY THE CONTRACTOR VIA WRITTEN REQUEST FOR INFORMATION (RFI). THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL. D. CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR ACCESS TO ALL ITEMS REQUIRING SPECIAL INSPECTION. ACCESS SHALL BE PROVIDED BY IN-PLACE LADDERS, SCAFFOLDS, LIFTS

AND/OR OTHER EQUIPMENT OPERATED BY THE CONTRACTOR'S PERSONNEL AS REQUIRED FOR SAFE OBSÉRVATION. INSPECTOR IS NOT RESPONSIBLE OR AUTHORIZED TO OPERATE CONTRACTOR'S UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF THEIR KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE

### 1. STEEL STUD. . UNPUNCHED STEEL STUD TRACK -ATTACHMENT REQUIRED 1/2" DIA. EXPANSION BOLTS AT 48" O.C. (2 MINIMUM) OR 3/8"x3/4" LONG POWDER DRIVEN PINS AT 12" O.C. FASTENER SHALL HAVE ICC REPORT FOR SAME INSTALLATION. MULTIPLE OR INTERSECTING STUDS WHERE OCCUR. CONNECT WITH 2 ROWS #10 S.M. SCREWS AT 24" O.C. STAGGERED SPACING, AT INTERSECTING STUDS 1 ROW OF #10 S.M. SCREWS EACH SIDE, EACH 2 #10 S.M. SCREWS 4 #10 S.M. SCREWS. #10 S.M. SCREWS AT 12" O.C. AT DOUBLE STUDS. STUD BLOCKING 9. 18 GA CLIP ANGLE BY STUD MFR WITH 3 #10 S.M. SCREWS AT EACH STUD WHERE SHOWN IN DETAILS. 10. TYP WELD CLIP TO FRAMING MEMBER OR AS NOTED IN DETAILS,

A. FOR SIZE AND GAGE OF TYPICAL STEEL STUDS, SEE GENERAL B. INSTALL ALL SCREWS PER STRUCTURAL NOTES. SPECIAL STEEL STUD MANUFACTURES SIZES ARE CALLED OUT ASSOC. SPECIFICATIONS. C. S.M. (SHEET METAL) SCREWS ON PLAN AND/OR DETAIL TYPICAL STEEL STUD WALL - SCREW CONNECTION 407-22 NO SCALE

SPLICE DETAIL

COLUMN TIES

BAR OFFSET

NOTES:

LAP - SEE G.S.N. MAXIMUM 1/5 LAP BUT NOT

MORE THAN 6".

- 1d (1" MINIMUM)

DEGREE HOOK.

4d (4" MINIMUM)

RADIUS=3d FOR BARS NOT

#18 BARS. 5d FOR ALL

GRADE 40 BARS WITH 180

12d (90 DEGREE HOOK).

1/2" PIN FOR #5 BARS

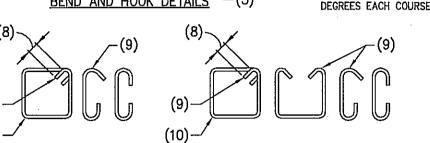
11. ROTATE TIE LOCATION 90

#11 BARS: 5d FOR #14 AND

WIRE TIES.

BAR CLEARANCE . 6d (4" MINIMUM). 135 DEGREE BEND. 10. BEND AROUND 1 1/2" PIN FOR

BEND AND HOOK DETAILS



TYPICAL REINFORCING BAR DETAILS 201-32 NO SCALE

BEAM STIRRUPS

	REBAR GRADE	MASONRY LAP SPLICE LENGTH					
REBAR SIZE (METRIC)		STEEL AT CENTER OF WALL			STEEL AT FACE OF WALL (& BOND BEAMS)		
		6" BLK	8" BLK	12" BLK	8" BLK	12" BLK	
<i>#</i> 4 (13)	40	20"	20"	20"	20"	20"	
#4 (13)	60	20"	20"	20"	20"	20"	
<i>#</i> 5 (16)	60	25"	25"	25"	25"	25"	
<b>#</b> 6 (19)	60	N/A	30"	30"	<b>30</b> "	30"	
<i>#</i> 7 (22)	60	N/A	35"	35"	N/A	35"	
#8 (25)	60	N/A	N/A	40"	N/A	40"	
SPLICE BARS AS OCCURS.							

<u>STEEL AT CENTER OF WALL</u> STEEL AT FACE OF WALL LAPS APPLY TO BOTH VERTICAL AND HORIZONTAL REINFORCING. PROVIDE BENT BARS TO MATCH HORIZONTAL BOND BEAM REINFORCING, AT CORNERS AND

WALL INTERSECTION TO MAINTAIN BOND BEAM CONTINUITY. 3. DO NOT SPLICE HORIZONTAL BARS WITHIN 8'-0" OF CONTROL JOINTS. FOR LADDER TYPE HORIZONTAL REINFORCING, SEE G.S.N. 5. LAP LENGTHS ARE BASED ON A CALCULATED STEEL STRESS (fs) NOT EXCEEDING 80 PERCENT

NOTES:

MASONRY WALL

(24" MINIMUM)

BOND BEAM REINFORCING

3. CORNER BARS SAME SIZE AND

SPACING AS HORIZONTAL

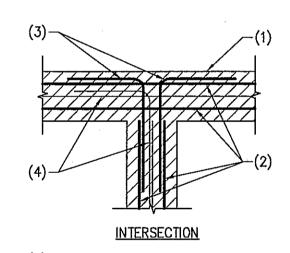
REINFORCING - LAP PER G.S.N.

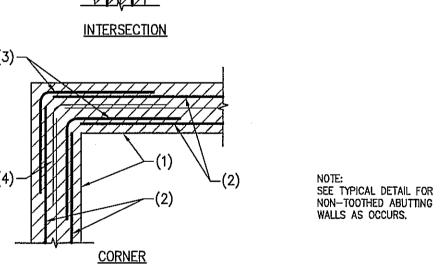
SINGLE BAR REINFORCING WHERE

OCCURS. ALTERNATE BEND OF

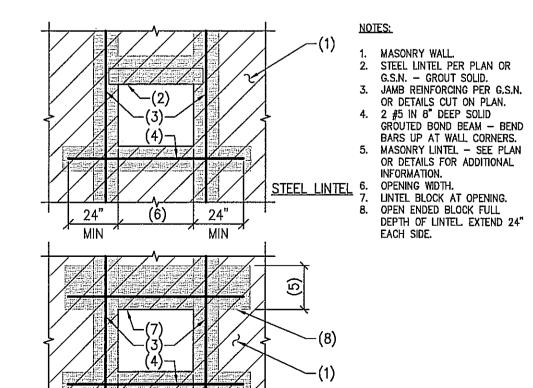
CORNER BARS AT INTERSECTION.

OF THE ALLOWABLE STEEL STRESS (Fs). MASONRY LAP SPLICES FOR REINFORCING STEEL - 2006 IBC 501-102 NO SCALE

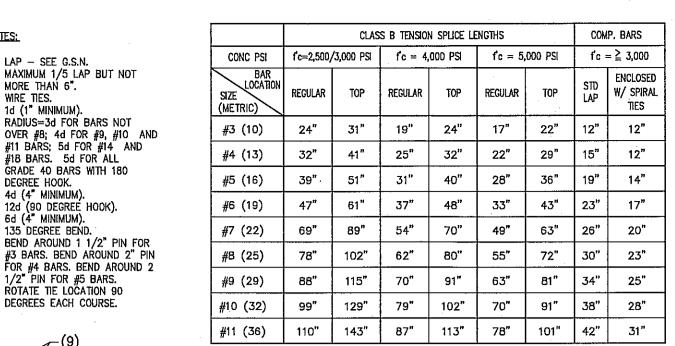




MASONRY BOND BEAM AT INTERSECTING WALLS NO SCALE



TYPICAL OPENING IN MASONRY WALL WITH STEEL OR MASONRY LINTEL



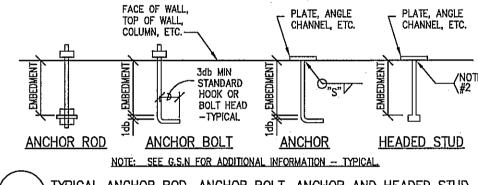
1. TOP BARS ARE ANY HORIZONTAL BARS PLACED SO THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCEMENT. 2. LAP SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318

UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS OR SCHEDULES. 3. CONTACT STRUCTURAL ENGINEER IF CLEAR SPACING OF REINFORCEMENT IS LESS THAN OR ≤ EQUAL TO 2 BAR DIAMETERS ( 2db), OR IF CLEAR COVER IS LESS THAN THE BAR DIAMETER THIS TABLE IS BASED ON NORMAL WEIGHT CONCRETE.

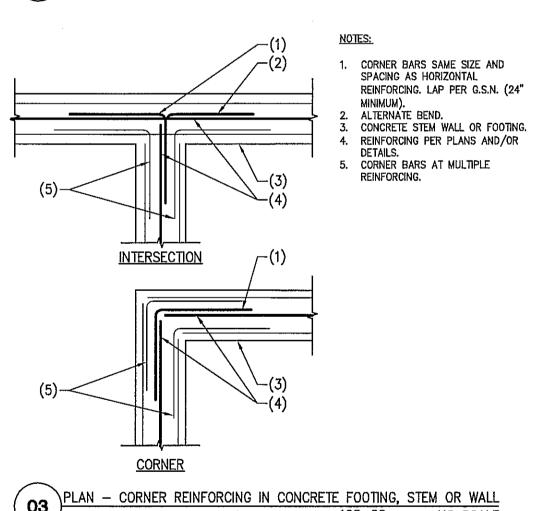
5. FOR ADDITIONAL INFORMATION, SEE G.S.N., PLANS, SCHEDULES AND DETAILS. LAP SCHEDULE FOR REINFORCING STEEL IN CONCRETE 201-31.01 NO SCALE

BOLT DIAMETER (db)	VERT BOLT EMBEDMENT LENGTH	HORIZ BOLT EMBEDMENT LENGTH	ANCHOR FILLET WELD SIZE, "S".
1/2"	6"	4"	1/4"
5/8"	6"	4"	5/16"
3/4"	7"	5"	5/16"
7/8 <sup>*</sup>	8"	6"	5/16"
1."	9"	7"	3/8"
1 1/8"	10"	8"	
1 1/4"	11"	9"	

1. PROVIDE ANCHORS, ANCHOR RODS AND ANCHOR BOLTS PER THIS SCHEDULE UNLESS NOTED OTHERWISE ON PLANS, SCHEDULES OR DETAILS. 2. AUTOMATIC WELDED TO DEVELOP FULL CAPACITY OF HIGH STRENGTH HEADED STUDS. 3. THICKNESS OF DRYPACK DOES NOT APPLY TOWARDS EMBEDMENT. 4. BOLT/PLATE EDGE CLEARANCES PER THE FOLLOWING - TYP U.N.O.: 3/4" DIA OR LESS - 1 1/4" 7/8" DIA ----- 1 1/2" 1" DIA ---- 1 3/4" 1 1/8" DIA ---- 2" 1 1/4" DIA ---- 2 1/4" OVER 1 1/4" DIA -- 1.75xDIA



TYPICAL ANCHOR ROD, ANCHOR BOLT, ANCHOR AND HEADED STUD SCHEDULE AND BOLT/PLATE EDGE CLEARANCES 401-02 NO SCALE



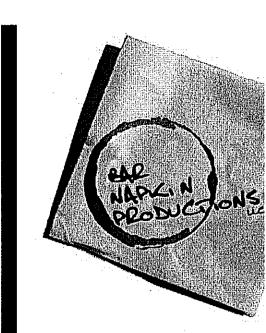
FOR ADDITIONAL INFORMATION SHOWN BUT NOT NOTED, SEE GENERAL STRUCTURAL NOTES ON SHEET S1.1 AND TYPICAL DETAIL SHEETS. THESE DRAWINGS/CALCULATIONS ARE CONSIDERED PRELIMINARY -NOT FOR CONSTRUCTION OR RECORDING UNLESS THE STRUCTURAL ENGINEER OF

RECORD'S SEAL IS AFFIXED WITH WRITTEN SIGNATURE.

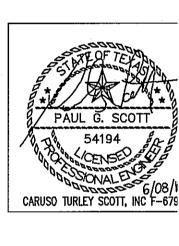
PROJECT ENGINEER APB PROJECT DRAFTER CARUSO - TURLEY - SCOTT - INC consulting structural engineers 1215 West Rio Salado Parkway, Suite 200

Tempe, Arizona 85281 (480) 774-1700 (774-1701 FAX) 발

PROJECT NUMBER 10-231 PROJECT MANAGER



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G.S.N.

TYPICAL DE

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